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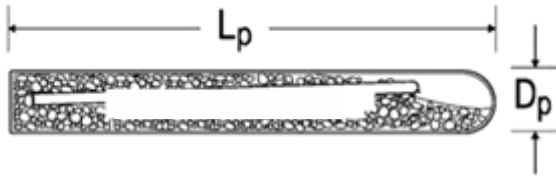


# SUPERCAP

Technical Datasheet  
for galvanised or stainless steel bonded anchor

**Product and installed condition**

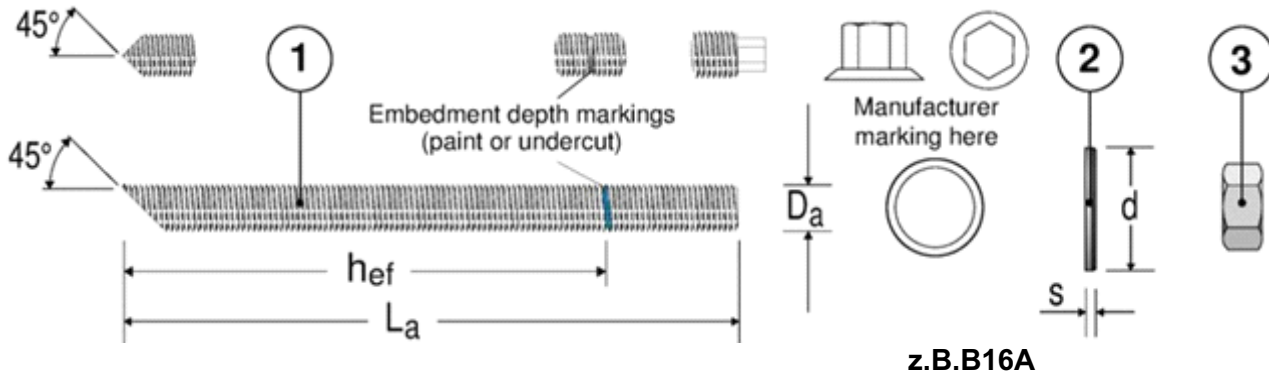
**Mortar Capsule SuperCap :**



**Marking capsule**

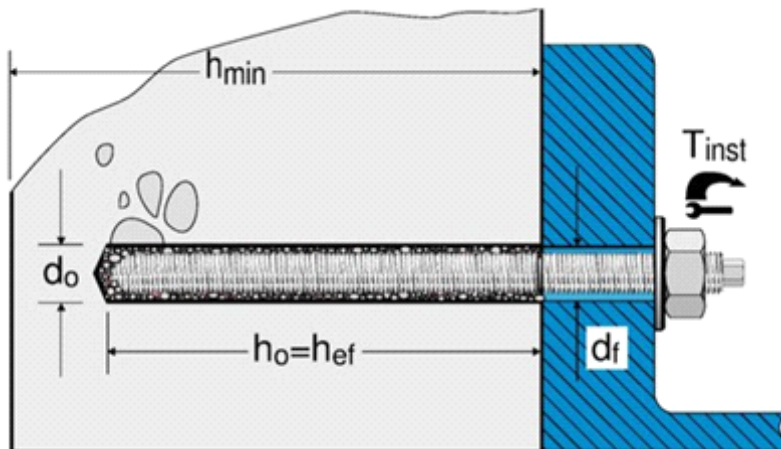
Manufacturer :	ICFS
Capsule Type :	SuperCap
Capsule Size :	M..

**Anchor Rod**



**Marking anchor rod**

<b>Manufacturer</b>	<b>B</b>		
<b>Size</b>	<b>8, 10, 12, 16, 20, 24</b>		
<b>Material</b>			
Galvanised property class 5.8	A	Stainless steel 1.4401, property class 70	C
Galvanised property class 8.8	B	Stainless steel 1.4404, property class 70	K
Hot dipped galvanised property class 5.8	H	Stainless steel 1.4529, property class 70	E
Hot dipped galvanised property class 8.8	I	Stainless steel 1.4565, property class 70	R
		Stainless steel 1.4571, property class 70	D
		Stainless steel 1.4401, property class 80	M
		Stainless steel 1.4404, property class 80	P
		Stainless steel 1.4571, property class 80	O



**Table A1: Materials**

Part	Description	Material			
1	Threaded rod	Carbon steel property class 5.8 or 8.8 EN ISO 898-1:2013		Stainless steel 1.4401, 1.4404 or 1.4571 property class A4-70 or A4-80 EN ISO 3506-1:2009 A <sub>5</sub> > 8% fracture elongation	High Corrosion resistant steel 1.4529 or 1.4565 property class 70 EN ISO 3506-1:2009 A <sub>5</sub> > 8% fracture elongation
		Galvanised steel ≥5µm acc. to EN ISO 4042:1999 A <sub>5</sub> > 8% fracture elongation	Hot dip galvanised steel EN ISO 10684:2004+AC:2009 A <sub>5</sub> > 8% fracture elongation		
2	Washer	Carbon steel		Stainless steel 1.4401, 1.4404 or 1.4571	High Corrosion resistant steel 1.4529 or 1.4565
		Galvanised steel ≥5µm acc. to EN ISO 4042:1999	Hot dip galvanised steel EN ISO 10684:2004+AC:2009		
		EN ISO 887:2006 oder EN ISO 7089:2000 bis EN ISO 7094:2000			
3	Hexagon nut	Carbon steel property class 5 to 8 EN ISO 898-2:2012		Stainless steel 1.4401, 1.4404 or 1.4571 property class A4-70 or A4 -80 EN ISO 3506-2:2009	High Corrosion resistant steel 1.4529 or 1.4565 property class 70 EN ISO 3506-2:2009
		Galvanised steel ≥5µm acc. to EN ISO 4042:1999	Hot dip galvanised steel EN ISO 10684:2004+AC:2009		
		EN ISO 4032:2012 oder EN ISO 4034:2012			
4	Glass capsule	Glass Quartz Resin Hardener			

**Table A2: Dimensions**

Part	Description	M8	M10	M12	M16	M20	M24
1	Threaded rod	D <sub>a</sub> [mm]	M8	M10	M12	M16	M24
		L <sub>a</sub> ≥ [mm]	95	100	120	140	190
2	Washer	s [mm]	1,6	2,1	2,5	3,0	4,0
		d [mm]	16	21	24	30	37
3	Hexagon nut	SW [mm]	13	17	19	24	36
4	Glass capsule	D <sub>p</sub> [mm]	9	11	13	17	24
		L <sub>p</sub> [mm]	80	80	95	95	175

## Specifications of intended use

### Anchorage subject to:

- Static and quasi-static loads: all sizes.

### Base materials

- Reinforced or unreinforced normal weight concrete without fibers according to EN 206:2013.
- Strength classes C20/25 to C50/60 according to EN 206-1 :2013.
- Non-cracked concrete.

### Temperature Range:

- I: - 40 °C to +40 °C (max long term temperature +24 °C and max short term temperature +40 °C)
- II: - 40°C to +80°C (max long term temperature +50 °C and max short term temperature +80 °C)

### Use conditions (Environmental conditions):

- Structures subject to dry internal conditions  
(zinc coated steel, stainless steel or high corrosion resistant steel).
- Structures subject to external atmospheric exposure (including industrial and marine environment) and to permanently damp internal condition, if no particular aggressive conditions exist  
(stainless steel or high corrosion resistant steel).
- Structures subject to external atmospheric exposure and to permanently damp internal condition, if other particular aggressive conditions exist  
(high corrosion resistant steel).

**Note:** Particular aggressive conditions are e.g. permanent, alternating immersion in seawater or the splash zone of seawater , chloride atmosphere of indoor swimming pools or atmosphere with extreme chemical pollution(e.g. in desulphurization plants or road tunnels where de-icing materials are used).

### Design:

- Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work.
- Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored. The position of the anchor is indicated on the design drawings (e. g. position of the anchor relative to reinforcement or to supports , etc.).
- Anchorages are designed in accordance with FprEN 1992-4:2016 and TR 055 .

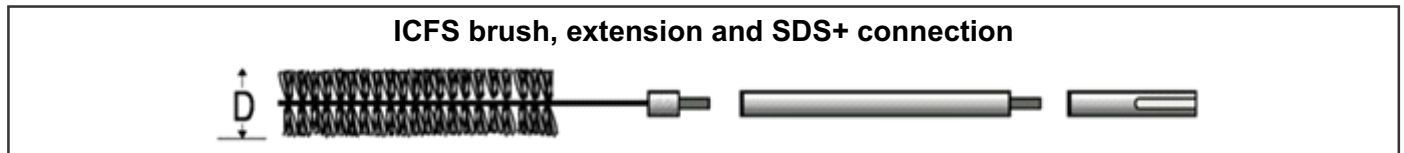
### Installation:

- Anchor installation carried out by appropriately qualified personnel and under the supervision of the person responsible for technical matters of the site.
- Dry or wet concrete: all sizes.
- Hole drilling by hammer drilling.
- cleaning the drill hole:  
removing possibly existing water in the drill hole completely and cleaning the drill hole by at least one blowing operation , by at least 1 x brushing | 1 x blowing | 1 x brushing operation by using the steel brush supplied by the manufacturer ; before brushing cleaning the brush and checking whether the brush diameter according to Annex B 2, Table 83 is still sufficient. The steel brush shall produce natural resistance as it enters the anchor hole. If this is not the case a new brush or a brush with a larger diameter must be used.

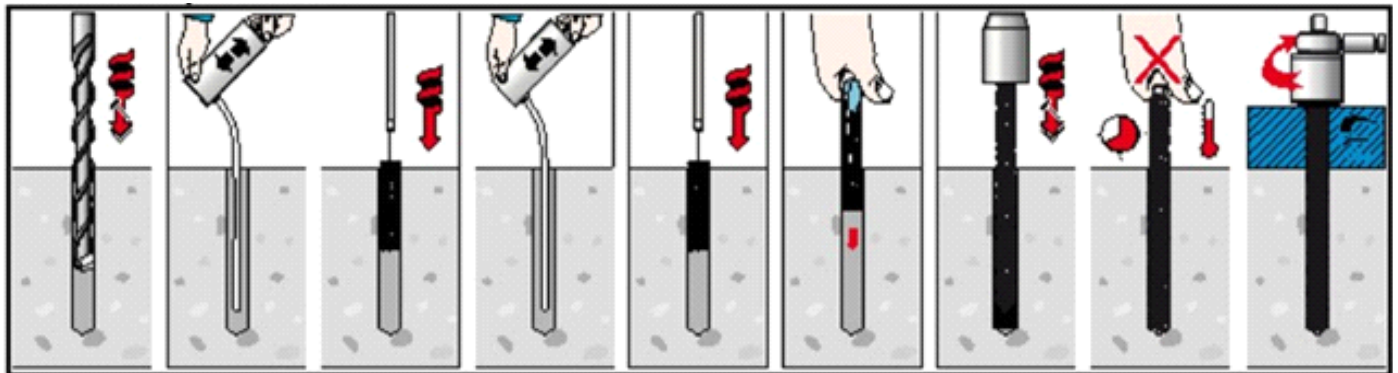
**Table B3 Installation parameters**

Anchor size			M8	M10	M12	M16	M20	M24
Nominal drill hole diameter	$d_0$	[mm]	10	12	14	18	25	28
Cutting diameter	$d_{cut}$	[mm]	10,5	12,5	14,5	18,5	25,5	28,5
Depth of drill hole	$h_0$	[mm]	80	90	110	125	170	210
Effective anchorage depth	$h_{ef}$	[mm]	80	90	110	125	170	210
Diameter of clearance hole in the fixture	$d_r$	[mm]	9	12	14	18	22	26
Diameter of steel brush	D	[mm]	11	13	16	20	27	30
Maximum torque moment	$T_{inst}$	[mm]	10	20	40	80	120	180

**Steel brush**



**Installation procedure**



**Table B2**

**Minimum member thickness, edge distance and spacing**

Anchor size			M8	M10	M12	M16	M20	M24
Minimum member thickness	$h_{min}$	[mm]	110	120	140	160	220	260
Minimum edge distance	$C_{min}$	[mm]	40	45	55	65	85	105
Minimum spacing	$S_{min}$	[mm]	40	45	55	65	85	105

**Table B3** Minimum curing time

Temperature in the concrete member	Minimum curing time in dry concrete	Minimum curing time in wet concrete
$\geq 0^{\circ}\text{C}$	5hrs.	10hrs.
$\geq +5^{\circ}\text{C}$	1hr.	2hrs.
$\geq +20^{\circ}\text{C}$	20min.	40min.
$\geq +30^{\circ}\text{C}$	10min.	20min.

**Table C1** Metal parts made of zinc plated or hot dip galvanised steel

Design method A, characteristic values for tension loads							
Anchor Sizes		M8	M10	M12	M16	M20	M24
<b>Steel failure</b>							
Characteristic resistance property class 5.8	$N_{Rk,s}$ [kN]	18	29	42	78	123	177
Characteristic resistance property class 8.8	$N_{Rk,s}$ [kN]	29	46	67	126	196	282
<b>Combined pull-out and concrete failure</b>							
Characteristic resistance in non-cracked concrete C20/25 to C50/60							
Temperature range I	$N_{Rk,p}^0$ [kN]	20	30	40	60	90	120
Temperature range II	$N_{Rk,p}^0$ [kN]	20	30	40	50	75	90
Factor for $k_1$	$k_{ucr,N}$ [-]	11,0					
<b>Concrete cone failure</b>							
Factor for $k_1$	$k_{ucr,N}$ [-]	11,0					
Characteristic edge distance	$C_{cr,N}$ [mm]	1,5 $h_{ef}$					
Characteristic spacing	$S_{cr,N}$ [mm]	3 $h_{ef}$					
<b>Splitting<sup>1)</sup></b>							
Edge distance	$C_{cr,sp}$ [mm]	1,5 $h_{ef}$	1 $h_{ef}$				
Spacing	$S_{cr,sp}$ [mm]	3 $h_{ef}$	2 $h_{ef}$				
Installation factor	$\gamma_{inst}$ [-]	1,2					

<sup>1)</sup>For the proof against splitting failure  $N_{Rk,c}^0$  has to be replaced by  $N_{Rk,p}^0$ .

**Table C2** Displacements under tension loads

Anchor Sizes		M8	M10	M12	M16	M20	M24
Tension load	N [kN]	8	12	16	20	30	38
Displacement	$\delta_{N0}$ [mm]	0,1	0,2	0,2	0,2	0,5	0,4
	$\delta_{N\infty}$ [mm]	0,5					



**Table C3** Metal parts made of stainless steel 1.4401, 1.4404 or 1.4571

Design method A, characteristic values for tension loads							
Anchor Sizes		M8	M10	M12	M16	M20	M24
<b>Steel failure</b>							
Characteristic resistance <b>strength class A4-70</b>	$N_{Rk,s}$ [kN]	26	40	59	110	172	247
Characteristic resistance <b>strength class A4-80</b>	$N_{Rk,s}$ [kN]	29	46	67	126	196	282
<b>Combined pull-out and concrete failure</b>							
Characteristic resistance in non-cracked concrete C20/25 to C50/60							
Temperature range I	$N_{Rk,p}^0$ [kN]	20	30	40	60	90	120
Temperature range II	$N_{Rk,p}^0$ [kN]	20	30	40	50	75	90
Factor for $k_1$	$k_{ucr,N}$ [-]	11,0					
<b>Concrete cone failure</b>							
Factor for $k_1$	$k_{ucr,N}$ [-]	11,0					
Characteristic edge distance	$C_{cr,N}$ [mm]	1,5 $h_{ef}$					
Characteristic spacing	$S_{cr,N}$ [mm]	3 $h_{ef}$					
<b>Splitting<sup>1)</sup></b>							
Edge distance	$C_{cr,sp}$ [mm]	1,5 $h_{ef}$	1 $h_{ef}$				
Spacing	$S_{cr,sp}$ [mm]	3 $h_{ef}$	2 $h_{ef}$				
Installation factor	$\gamma_{inst}$ [-]	1,2					

<sup>1)</sup>For the proof against splitting failure  $N_{Rk,c}^0$  has to be replaced by  $N_{Rk,p}^0$ .

**Table C4** Displacements under tension loads

Anchor Sizes		M8	M10	M12	M16	M20	M24
Tension load	N [kN]	8	12	16	20	30	38
Displacement	$\delta_{N0}$ [mm]	0,1	0,2	0,2	0,2	0,5	0,4
	$\delta_{N\infty}$ [mm]	0,5					

**Table C5** Metal parts made of high corrosion resistant steel 1.4529 or 1.4565

Design method A, characteristic values for tension loads							
Anchor Sizes		M8	M10	M12	M16	M20	M24
<b>Steel failure</b>							
Characteristic resistance strength class 70	$N_{Rk,s}$ [kN]	26	40	59	110	172	247
<b>Combined pull-out and concrete failure</b>							
Characteristic resistance in non-cracked concrete C20/25 to C50/60							
Temperature range I	$N_{Rk,p}^0$ [kN]	20	30	40	60	90	120
Temperature range II	$N_{Rk,p}^0$ [kN]	20	30	40	50	75	90
Factor for $k_1$	$k_{ucr,N}$ [-]	11,0					
<b>Concrete cone failure</b>							
Factor for $k_1$	$k_{ucr,N}$ [-]	11,0					
Characteristic edge distance	$C_{cr,N}$ [mm]	1,5 $h_{ef}$					
Characteristic spacing	$S_{cr,N}$ [mm]	3 $h_{ef}$					
<b>Splitting<sup>1)</sup></b>							
Edge distance	$C_{cr,sp}$ [mm]	1,5 $h_{ef}$	1 $h_{ef}$				
Spacing	$S_{cr,sp}$ [mm]	3 $h_{ef}$	2 $h_{ef}$				
Installation factor	$\gamma_{inst}$ [-]	1,2					

For the proof against splitting failure  $N_{Rk,c}^0$  has to be replaced by  $N_{Rk,p}^0$

**Table C6** Displacements under tension loads

Anchor Sizes		M8	M10	M12	M16	M20	M24
Tension load	$N$ [kN]	8	12	16	20	30	38
Displacement	$\delta_{N0}$ [mm]	0,1	0,2	0,2	0,2	0,5	0,4
	$\delta_{N\infty}$ [mm]	0,5					



**Table C7** Metal parts made of zinc plated or hot dip galvanised steel

Design method A, characteristic values for shear loads							
Anchor Sizes		M8	M10	M12	M16	M20	M24
<b>Steel failure without lever arm</b>							
Characteristic resistance property class 5.8	$V_{RK,S}^0$ [kN]	9	14	21	39	61	88
Characteristic resistance property class 8.8	$V_{RK,S}^0$ [kN]	15	23	33	63	98	141
Ductility factor	$k_7$ [-]	0,8					
<b>Steel failure with lever arm</b>							
Characteristic resistance property class 5.8	$M_{RK,S}^0$ [Nm]	19	37	65	166	325	561
Characteristic resistance property class 8.8	$M_{RK,S}^0$ [Nm]	30	60	105	266	519	898
<b>Pry out failure</b>							
Factor	$k_8$ [-]	2,0					
Installation factor	$\gamma_{inst}$ [-]	1,0					
<b>Concrete edge failure</b>							
Effective length of anchor	$l_f$ [mm]	80	90	110	125	170	210
Outside diameter of anchor	$d_{nom}$ [mm]	10	12	14	18	25	28
Installation factor	$\gamma_{inst}$ [-]	1,0					

**Table C8** Displacements under shear loads

Anchor Sizes		M8	M10	M12	M16	M20	M24
Shear load	$V$ [kN]	5	8	12	22	35	50
Displacement	$\delta_{v0}$ [mm]	2	3	3	4	5	5
	$\delta_{v\infty}$ [mm]	4	5	5	6	7	7

**Table C9** Metal parts made of stainless steel 1.4401, 1.4404 or 1.4571

Design method A, characteristic values for shear loads							
Anchor Sizes		M8	M10	M12	M16	M20	M24
<b>Steel failure without lever arm</b>							
Characteristic resistance <b>strength class A4-70</b>	$V_{RK,S}^0$ [kN]	13	20	29	55	86	124
Characteristic resistance <b>strength class A4-80</b>	$V_{RK,S}^0$ [kN]	15	23	33	62	98	141
Ductility factor	$k_7$ [-]	0,8					
<b>Steel failure with lever arm</b>							
Characteristic bending moment <b>strength class A4-70</b>	$M_{RK,S}^0$ [Nm]	26	52	92	233	454	785
Characteristic bending moment <b>strength class A4-80</b>	$M_{RK,S}^0$ [Nm]	30	60	105	266	519	898
<b>Pry out failure</b>							
Factor	$k_8$ [-]	2,0					
Installation factor	$\gamma_{inst}$ [-]	1,0					
<b>Concrete edge failure</b>							
Effective length of anchor	$l_f$ [mm]	80	90	110	125	170	210
Outside diameter of anchor	$d_{nom}$ [mm]	10	12	14	18	25	28
Installation factor	$\gamma_{inst}$ [-]	1,0					

**Table C10** Displacements under shear loads

Anchor Sizes		M8	M10	M12	M16	M20	M24
Shear load	$V$ [kN]	5	8	12	22	35	50
Displacement	$\delta_{v0}$ [mm]	2	3	3	4	5	5
	$\delta_{v\infty}$ [mm]	4	5	5	6	7	7

**Table C11** Metal parts made of high corrosion resistant steel 1.4529 or 1.4565

Design method A, characteristic values for shear loads							
Anchor Sizes		M8	M10	M12	M16	M20	M24
<b>Steel failure without lever arm</b>							
Characteristic resistance <b>strength class 70</b>	$V_{RK,S}^0$ [kN]	13	20	29	55	86	124
Ductility factor	$k_7$ [-]	0,8					
<b>Steel failure with lever arm</b>							
Characteristic bending moment <b>strength class 70</b>	$M_{RK,S}^0$ [Nm]	26	52	92	233	454	785
<b>Pry out failure</b>							
Factor	$k_8$ [-]	2,0					
Installation factor	$\gamma_{inst}$ [-]	1,0					
<b>Concrete edge failure</b>							
Effective length of anchor	$l_f$ [mm]	80	90	110	125	170	210
Outside diameter of anchor	$d_{nom}$ [mm]	10	12	14	18	25	28
Installation factor	$\gamma_{inst}$ [-]	1,0					

**Table C12** Displacements under shear loads

Anchor Sizes		M8	M10	M12	M16	M20	M24
Shear load	$V$ [kN]	5	8	12	22	35	50
Displacement	$\delta_{v0}$ [mm]	2	3	3	4	5	5
	$\delta_{v\infty}$ [mm]	4	5	5	6	7	7

CHANNEL PARTNER



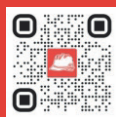
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